An Assessment on the Existing Flood Mitigation System in Village East Executive Homes, Cainta, Rizal Using the Rational Method

A Research Presented to Faculty of Civil Engineering
College of Engineering, Architecture, and Technology
De La Salle University – Dasmariñas
Dasmariñas City

In Partial Fulfillment of Requirements for the Course

Bachelor of Science in Civil Engineering

Submitted by:

Wellabelle M. Cuarto

Josen Brian O. Laserna

Nizza Mariel O. Zacarias

CEE51

October 2011

Abstract

This study was intended to assess an existing flood mitigation system for the Village East Cainta Rizal by using the rational method. For this purpose, the researchers gathered data such as rainfall intensity data, location, topographical and land use data and made use of empirical models. Based from the computations made by the researchers, the existing drainage system of the village is unable to accommodate the entire runoff entering the village. The results of the computations presented, showing the flow velocity, flow capacity and the accumulated flow of the existing design can help the Homeowners' Association of Village East Cainta Rizal to further improve their drainage system. They can be also helped in their planning activities relating to the community-level flood-mitigation and disaster response. A program of pipe sizes and discharge flows are also available for use as a guide for their maintenance of their drainage canals.

.

Table of Contents

		Page No.
Approval S	Sheet	ii
Abstract		iii
Acknowled	dgement	iv
Chapter 1	The Problem and Its Background	
1.1	Introduction	1
1.2	Statement of the Problem	2
1.3	Research Objectives	
1.4	Scope and Limitations	3
1.5	Significance of the Study	4
1.6	Study Framework	
1.7	Definition of Terms	6
Chapter 2	Review of Related Literature	
2.1	Project Description	
2.2	Return Period	
2.3	Rational Method and Surface Runoff	
2.4	Precipitation	
2.5	Rainfall Intensity	11
2.6	Time of Concentration	12
2.7	Seelye Chart Method	
2.8	Kirpitch Chart	13
2.9	Minor Losses in Pipelines	
2.10	The Hazen – Williams Equation	15
2.11	Uniform Flow and Continuity equation	17
2.12	Manning's Equation	17
2.13	Coefficient of imperviousness	
2.14	Manhole Spacing	19

2.15	Design Standards and Considerations for Storm
	Drain Pipe Systems
2.16	Commercial Reinforced Concrete Pipe Sizes
Chapter 3	Methodology
3.1	Analytical Framework
3.2	Data Gathering
3.3	Data Analysis
Chapter 4	Data Presentation and Analysis
4.1	Data Presentation
4.	1.1 Summary of Adapted Parameters
4.	1.2 Summary of Peak <mark>Ru</mark> noff
4.	1.3 Summary of Pe <mark>ak Flow</mark> and Maximum Capacities 27
4.2	Data analysis
4.3	Data Results
Chapter 5	Conclusion and Recommendation
5.1	Conclusion
5.2	Recommendations
Appendices	s31
Appe	ndix A. Maps, Plans and Charts used
Appe	endix B. Secondary Data and Results
Арре	endix C. Sample Computations
Appe	ndix D. Documentation (photos)
Apper	ndix E. Approved Letters
Bibliograph	า y

List of Tables and Figures

Table No.	Title	Page No.
2.1	Runoff Coefficient for Q _p =CiA equation	10
2.2	Rainfall Intensity Classification	
2.3	Minor Losses in Pipelines	16
2.4	Values of Hazen – Williams Coefficient	
2.5	Manning's Roughness Coefficient	
2.6	Impervious Coefficient Values	
4.1	Adapted Standards and Design Parameters	25
4.2	Summary of Peak Runoff	
4.3	Summary of Peak Flow and Maximum Capacities	28
4.4	Summary of Data Results	
6.1	Summary of Peak Flow and Maximum	
	Capacities (English Unit)	47
6.2	Computation for Average Runoff	48
6.3	Area Computation	49
6.4	Sample Computations for a pipe network	
	of Granada Street	57
6.5	Flow Capacity Computation (Drainage A)	
6.6	Flow Capacity Computation (Drainage B)	
6.7	Computation for Resizing (Drainage A)	
6.8	Computation for Resizing (Drainage B)	79
Figure No.	Title	Page No.
1.1	Study Framework of the Study	5
2.1	Topographical Map of Area Considered	
2.2	Seelye Chart	

2.3	Kirpitch Chart
3.1	Methodological Method
4.1	Recommended Pipe resizing for Drainage A 30
4.2	Recommended Pipe resizing for Drainage B
6.1	Topographical Map of the Area to be considered
6.2	Drainage Plan of Village East Executive Homes
6.3	Drainage water flow in Village East Executive Homes 37
6.4	Map of Flooded Areas
6.5	Corrections on Relation Based on the Actual
	Topographical Survey
6.6	Areal Computation Figure
6.7	Flow Paths/Hydraulic Lengths
6.8	Seelye Chart for Computation of Travel
	Time Overland Flow
6.9	Diagram for Computing the Average Velocity of Shallow
	Concentrated Flow
6.10	Kirpitch Chart
7.1	Pipe system of Granada Street
7.2	Definition Sketch for pipe flow
8.1	Hydraulic gradient familiarization in
	DPWH-Masinag Antipolo City
8.2	Gathering data for rainfall intensity from PAGASA 80
8.3	Village East Executive Homes Association's
	Office and Site Map
8.4	Roads along Drainage A and Drainage B respectively
8.5	Verification of drainage size in pipe line of drainage B
	along Cougar St (18" Ø pipe)
8.6	Outflow point of drainage A and B respectively 82
8.7	Drainage A along Village East Avenue 83
8.8	Drainage A for Daytona St. and Escort St. respectively 83
8.9	Drainage A for Bentley St. and BMW St. respectively 84

8.10	Drainage A along Village East Avenue corner of Corona St.
	and Corolla St. respectively
8.11	Drainage B for Crown St. and Corona St. respectively 85
8.12	Drainage B for Cougar St. and Celica St. respectively 85
8.13	Drainage B for Charger St. and Champ St. respectively 86
8.14	Drainage B for Corolla St. and Citroen St. respectively 86
8.15	Drainage B for Caprice St. and Cobra St. respectively 87
8.16	Drainage B for Courier St. and Charger St. respectively 87
8.17	Shot A (corner of Corona St. and Village East Drive) 88
8.18	Shot B (near BMW Street)
8.19	Shot C (corner of Daytona St. and Dasher St.) 89
8.20	Shot D (corner between Escort St. and Crown St.) 89
8.21	Shot E (corner near Ferrari St. and Vista Verde Ext.) 90
8.22	Shot F (corner of Fiero and Fiat St.)